

Research Article

LNAPL migration through soil cement barrier with and without flow condition

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This paper was originally presented at the International Conference on the Role of Universities in Hands-On Education, Chiang Mai, Thailand, August 2009.

Abstract

The spill of hydrocarbons from industrial plants is a significant problem that affects groundwater. Contaminant migration can widely spread in the subsurface by advection and diffusion. The effect of contaminated groundwater becomes more serious if contamination occurs in sandy soil. This paper focuses on the study of light non aqueous phase liquid (LNAPL) migration in soil and through a containment barrier. The simulation study of contaminant migration considers 2 scenarios as follows: (1) without groundwater flow and (2) with groundwater flow with a hydraulic gradient of 0.017. The wall, 5 m deep and 1 m thick, was modelled as a containment system. The NAPL spill was modelled with a constant rate release lasting 2 years. The study found that the permeability of soil and the hydraulic gradient of the aquifer were the factors that most affected the contaminant migration. The results obtained could be used as a guide for the design of impervious wall dimensions and properties to properly contain contaminant migration.

Keywords: Hydrocarbons, LNAPL migration, Contamination, Hydraulic gradient, Containment

Introduction

Subsurface contamination problems due to the release of toxic substances such as inorganic and organic compounds including hydrocarbon volatile organic compounds (VOCs) may affect the environment and the life cycle of natural animals and humans.

The spill of light non aqueous phase liquid (LNAPL), such as gasoline, into the vadose zone is more risky than the spill of the heavy contaminant (DNAPL) because LNAPL can spread quickly, especially in the presence of high permeability soil. For these reasons, this paper focuses on the benzene (STD) migration behaviour through a soil cement barrier. Benzene is an aromatic hydrocarbon having a high solubility in water and a non negligible vapour pressure. When spilt into the subsurface it migrates, giving rise to multiphase flow processes.

In this study, the simulations took into account different barrier materials and different aquifer hydraulic gradients. The TMVOC simulator was used within the PetraSim 4.2 pre- and post-processing interface. PetraSim is one of the graphical interfaces available for the TOUGH2 family of reservoir simulators developed at Lawrence Berkeley National Laboratory (USA). TOUGH2 and its derivatives were recognized for their broad range of subsurface simulation capabilities, including heat and multiphase flow and reactive transport [1, 2, 3]. In the past, modelling of multiphase organic contaminant migration was performed by several authors, such as Abriola and Pinder [4], Kaluarachchi and Parker [5], Falta *et al.* [1], Pruess and Battistelli [2], Fagerlund and Niemi [6] and Battistelli [7].

Soil-cement walls are structures often used to improve the geotechnical properties of soft soil. They can be constructed by two methods as follows: (1) rotary mixed method, which is the technique preferred for cohesive soil, with widespread use in Japan and (2) jet grouting method, which is the technique for both cohesive soil and cohesionless soil. The latter method can especially be used for sandy soil where the injection of cement slurry is more effective than in clay. This approach offers the advantage of building wall columns in both vertical and inclined direction by cement based grout. The construction of soil-cement columns by means of jet grouting can be depicted step-by-step as shown in Figure 1.

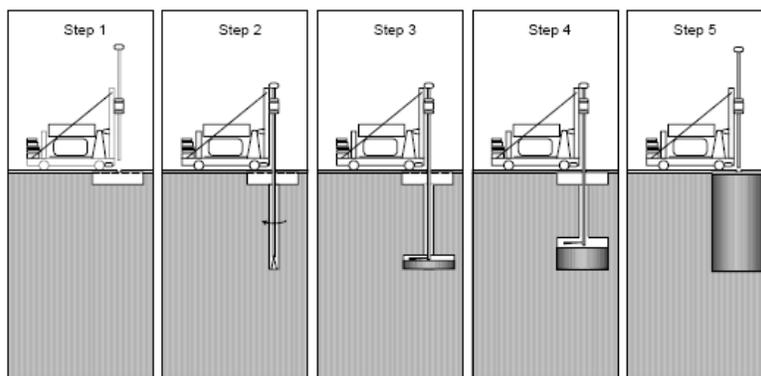


Figure1. Step-by-step construction of soil cements columns by means of jet grouting technique.

Research Methodology

Model characteristics and material properties

The conceptual models used in the study are shown in Figure 2. They are two dimensional sections 60.2 m long, 15.1 m thick and 1 m wide. The characteristics of the four models are: (1) no groundwater flow (hydraulic gradient equal to zero), (2) groundwater flow with a hydraulic gradient of 0.017 (water table difference of 1 m along a distance of 60 m (1/60)), (3) no groundwater flow with containment (hydraulic gradient equal to zero), and (4) groundwater flow with a hydraulic gradient of 0.017 with containment. The spill point of the benzene is located in the unsaturated zone at a distance from the left side of 29.6 m for models 1 and 3 and 19.6 m for models 2 and 4. The groundwater table is 2 m below the ground surface for models 1 and 3, while for models 2 and 4, it is 2 m and 3 m deep at the left and right boundaries, respectively. The containment system is 1 m thick, 5 m deep and is located at 1.5 m from the spill point in the left and right direction.

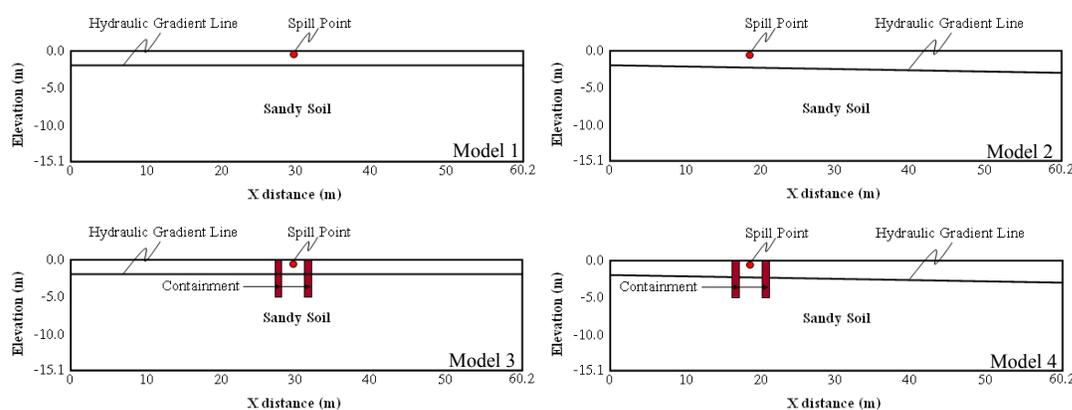


Figure 2. Conceptual models 1, 2, 3 and 4.

Effect of intrinsic permeability was studied. Three intrinsic permeability values of soil were used which are 10^{-9} m^2 , 10^{-10} m^2 and 10^{-11} m^2 . The intrinsic permeability of barrier value was also indicated which are 10^{-13} m^2 , 10^{-14} m^2 and 10^{-15} m^2 . This study considers a total of 24 different cases; basic petrophysical properties are listed in Table 1. The relative permeability and capillary pressure curves for three-phase systems are described according to the Stone (1970) and Parker et al. (1987) models, respectively [4]. The corresponding parameters are summarized in Tables 2 and 3 for the relative permeability and the capillary pressure, respectively. The simulations are performed at a constant temperature of 20°C . The atmospheric boundary conditions are fixed at the grid top and specified as constant absolute pressure of $1.01 \times 10^5 \text{ Pa}$.

Table 1. Main petrophysical properties of rock domains.

Soil Criteria	Rock Grain Density kg m ⁻³	Porosity	Horizontal Permeability m ²	Vertical Permeability m ²
ATMOS	2600	0.35	1x10 ⁻⁸	1x10 ⁻⁸
Soil 1	2600	0.31	1x10 ⁻⁹	1x10 ⁻⁹
Soil 2	2600	0.35	1x10 ⁻¹⁰	1x10 ⁻¹⁰
Soil 3	2600	0.39	1x10 ⁻¹¹	1x10 ⁻¹¹
Wall 1	2600	0.43	1x10 ⁻¹³	1x10 ⁻¹³
Wall 2	2600	0.47	1x10 ⁻¹⁴	1x10 ⁻¹⁴
Wall 3	2600	0.51	1x10 ⁻¹⁵	1x10 ⁻¹⁵

Table 2. Relative permeability parameters of different rock domains (first Stone’s modified model).

Soil criteria	Swr	Snr	Sgr	n exponent
ATMOS	0.15	0.05	0.05	3
Soil1, Wall1	0.15	0.05	0.05	3
Soil2, Wall2	0.15	0.05	0.05	3
Soil3, Wall3	0.15	0.05	0.05	3

Remarks: Swr = irreducible aqueous phase saturation, Snr = irreducible NAPL saturation, Sgr = irreducible gas phase saturation, NAPL = non aqueous liquid

Table 3. Capillary pressure parameters of different rock domains (Parker’s model).

Soil criteria	Sm	αgn	αnw	n exponent
ATMOS		no capillary		
Soil 1	0	100	110	1.84
Soil 2	0	30	33	1.84
Soil 3	0	10	11	1.84
Wall 1	0	1	1.1	1.84
Wall 2	0	3	3.3	1.84
Wall 3	0	0.1	0.11	1.84

Remarks: Sm = limiting saturation, αgn = strength parameter for gas-NAPL, αnw = strength parameter for NAPL-aqueous phase liquid

The applied boundary conditions are shown in Table 4. For this application, the formation of heterogeneities, the seasonal water table fluctuations, and the water infiltration have been neglected.

Table 4. Boundary conditions applied to simulation.

Boundary	Pressure	Condition
Hydraulic Gradient, $i = 0$		
Top	1.01×10^5	Gas Only
Left	1.01×10^5	Gas and Water, Above Water Table ($z \leq 2.1$ m, water sat. = 0.20)
($x = 0$ m)	$1.01 \times 10^5 + 9789z$	Water Only, Below Water Table ($z > 2.1$ m)
Right	1.01×10^5	Gas and Water, Above Water Table ($z \leq 2.1$ m)
($x = 60.2$ m)	$1.01 \times 10^5 + 9789z$	Water Only, Below Water Table ($z > 2.1$ m)
Boundary	Pressure	Condition
Hydraulic Gradient, $i = 0.017$		
Top	1.01×10^5	Gas Only
Left	1.01×10^5	Gas and Water, Above Water Table ($z \leq 2.1$ m, water sat. = 0.20)
($x = 0$ m)	$1.01 \times 10^5 + 9789z$	Water Only, Below Water Table ($z > 2.1$ m)
Right	1.01×10^5	Gas and Water, Above Water Table ($z \leq 3.1$ m)
($x = 60.2$ m)	$1.01 \times 10^5 + 9789z$	Water Only, Below Water Table ($z > 3.1$ m)

Simulated scenarios

The modelling is discretized with 16 layers and 62 columns for a total of 992 elements. The vertical and horizontal spacing is 1x1 m, except the elements of the top row which are 1x0.1 m; left and right boundary columns have the spacing of 0.1x1 m. The simulations are divided into several steps as follows: (1) setting up the initial conditions at left and right boundary columns; (2) run to steady state controlled by gravity and capillary forces and subjected to the boundary conditions at lateral and top grid sides specified for each case; and (3) modelling of spill for 2 years starting from the steady state conditions obtained at step 2. The LNAPL spill has been modelled assuming a constant rate of 1.154×10^{-5} kg/s, equivalent to 1 kg/day. In this study, the effectiveness of the barrier is analyzed looking at the effects of aquifer permeability and hydraulic gradient.

Results and Discussion

Modelling of steady state

The initial conditions to model the spill scenarios were obtained running the system to steady state governed by gravity and capillary forces under the boundary conditions specified for each case. The steady state pressure distribution is shown in Figure 3. In case of $i = 0$, the LNAPL plume spreads symmetrically over the water table, while in the case of aquifer flow, the LNAPL plume moves preferentially following the water table slope.

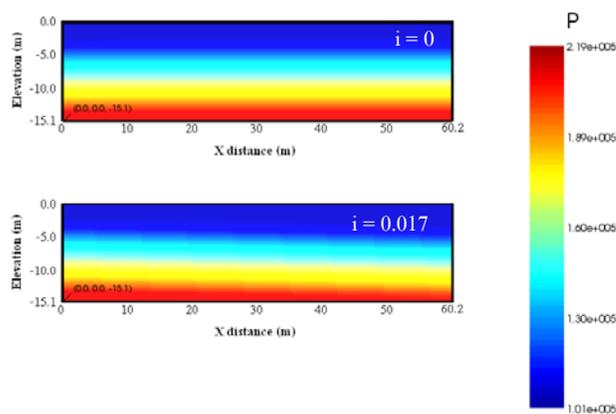


Figure 3. Pressure distribution at steady state conditions.

Migration of LNAPL into subsurface

The spill of benzene is modelled at a constant rate of 1 kg/day into the vadose zone for 2 years. The simulations are performed under isothermal conditions. In this study, the standard benzene properties (STD) supplied by Petrasim have been used. The diffusion coefficients of mass components in the different phases are summarized in Table 5.

Table 5. Molecular diffusion coefficients of mass components.

	GAS	AQUEOUS	NAPL
Air	2.0×10^{-5}	6.0×10^{-10}	6.0×10^{-10}
Water	2.0×10^{-5}	6.0×10^{-10}	6.0×10^{-10}
Benzene	7.7×10^{-6}	6.0×10^{-10}	6.0×10^{-10}

The simulation results relative to the total mass fraction of benzene in the aqueous liquid (XVOCW) can be described as follows:

- (1) Case $i = 0$: the benzene moves downward according to gravity; then the LNAPL plume floats on the water table and spreads out laterally. The depth reached by the dissolved benzene plume below the water table is about 1.5 m and the distance of the benzene migration decreases with the soil permeability. In the presence of a containment wall the dissolved benzene is contained by the barrier and cannot migrate outside the containment.
- (2) Case of $i = 0.017$: once reaching the water table, the LNAPL plume migrates preferentially in the direction of the water table gradient. The shape of the dissolved benzene plume changes depending on the permeability of soil. The result of the model scenario without containment shows that if permeability of soil decreases, the LNAPL plume could migrate to longer distances. Higher soil permeability allows a greater evaporation of benzene. The results of the model scenario with wall containment show that the benzene migration is reduced by the containment. The dissolved benzene plume moves downward along the barrier and some benzene can flow under the wall base when the soil has lower permeability as shown in Figure 4.

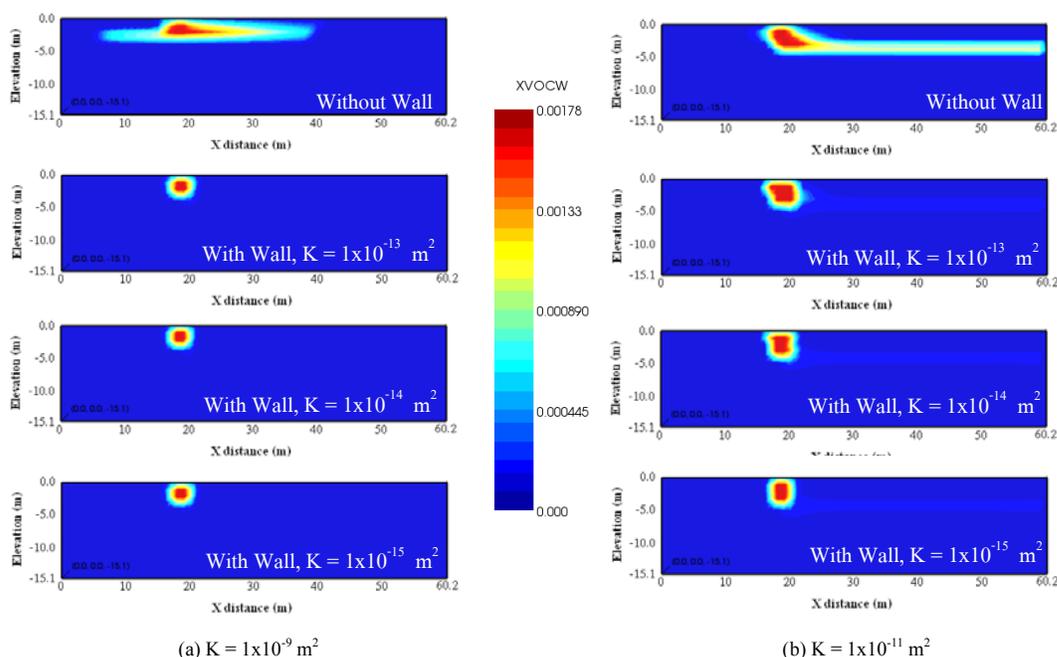
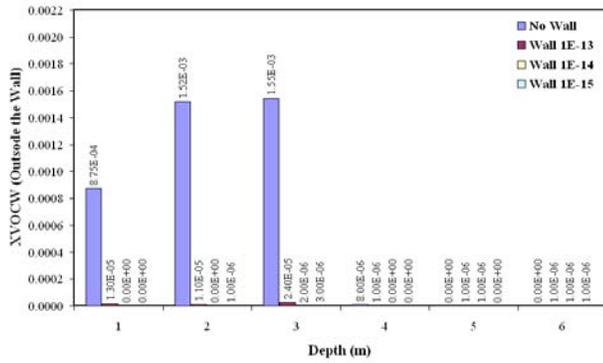
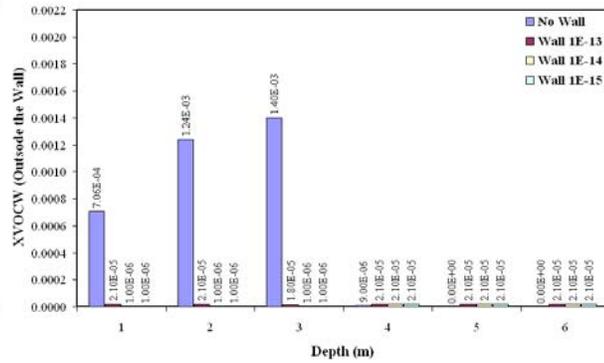


Figure 4. Total mass fraction of VOCs in aqueous phase in sandy soil with hydraulic gradient, $i = 0.017$.

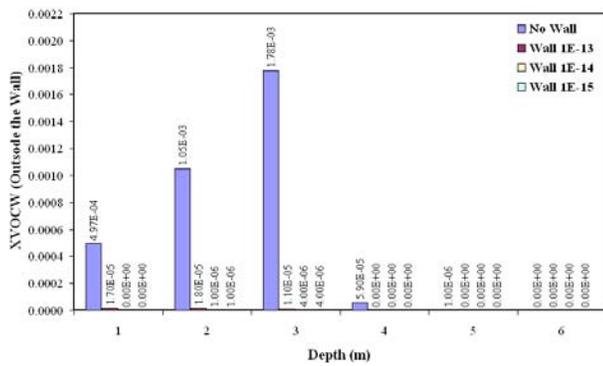
The total mass fraction of VOCs dissolved in the aqueous phase outside the containment zone at depth of 1 – 6 m below the ground surface in the case of $i = 0$ where containment is reduced close to zero, as shown in Figures 6a – 6c. For the case of $i = 0.017$, the concentration of the benzene increases at the end of the barrier according to permeability decrease due to the effect of the groundwater flow as shown in Figures 6d – 6f.



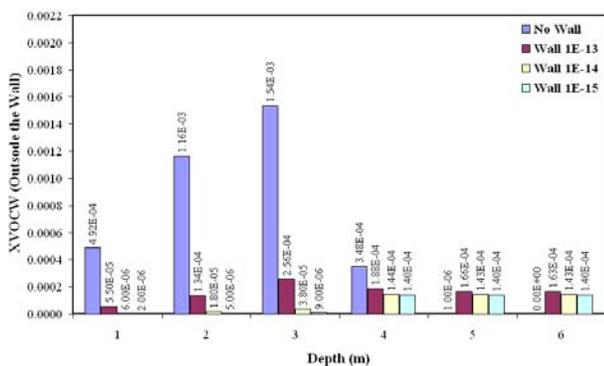
(a) $K = 1 \times 10^{-9} \text{ m}^2$ and $i = 0$



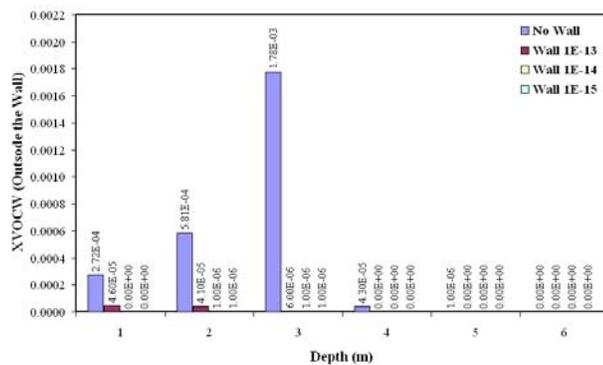
(d) $K = 1 \times 10^{-9} \text{ m}^2$ and $i = 0.017$



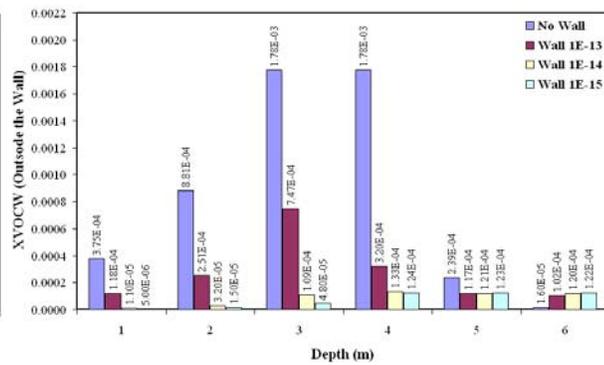
(b) $K = 1 \times 10^{-10} \text{ m}^2$ and $i = 0$



(e) $K = 1 \times 10^{-10} \text{ m}^2$ and $i = 0.017$



(c) $K = 1 \times 10^{-11} \text{ m}^2$ and $i = 0$



(f) $K = 1 \times 10^{-11} \text{ m}^2$ and $i = 0.017$

Figure 6. Comparison of Total Mass Fraction of VOCs in Aqueous Phase outside the wall for sandy soil layer.

Conclusions

This paper presents simulation of benzene migration in the subsurface as a consequence of a constant rate spill in the unsaturated zone. Several scenarios have been modelled with a phreatic aquifer in a sandy soil of varying permeability, with different hydraulic gradients and with or without the presence of a vertical containment wall. Simulation results reveal that the soil-cement barrier can reduce the contamination of the benzene and show that soil permeability and water table hydraulic gradient are the significant factors.

The benzene migration in the case of $i = 0$, only occurs by gravity driven NAPL plume flow and diffusion of dissolved benzene in the groundwater. In the presence of aquifer flow, dissolved benzene may be transported over long distances by advective flow. Without the groundwater flow, the contaminant migration is contained by the soil-cement barrier in the vadose zone; the dissolved benzene plume reaches less than 2 m below the water table. Consequently, the depth of the soil-cement wall should be more than 2 m below the ground water level. With groundwater flow, the concentration of contamination depends on the hydraulic gradient which enhances the transport processes. The hydraulic gradient has an impact on the depth of contaminant migration outside the wall. From the scenarios simulated, it can be concluded that soil-cement barriers can be used to limit the spread of benzene spilled in the unsaturated zone. Modelling studies such as that described may help in the design of containment operations and in risk assessment studies.

The work presented is a preliminary study dealing with the processes controlling the migration of VOCs spilled in the vadose zone in the presence of vertical containment walls in sandy aquifers. The properties of the soil-cement used in these simulations are derived from bibliographic sources. Experimental derived properties will be considered in future simulation work.

Acknowledgements

The authors would like to thank the Commission on Higher Education of Thailand for support given through a grant fund under the Strategic Scholarships for Frontier Research Network for the Joint Ph.D. Program, Thai Doctoral degree. Thanks are also due to Dr. Alfredo Battistelli for information about the TMVOC simulator and for valuable suggestions.

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